

CHAPTER

Prentice Hall Structural Steel Design LRFD Method Third Edition

UNIVERSITY OF MARYLAND COLLEGE PARK

# INTRODUCTION TO BEAMS

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Part II – Structural Steel Design and Analysis

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8d

Prentice Hall

CHAPTER 8d. INTRODUCTION TO BEAMS Slide No. 1

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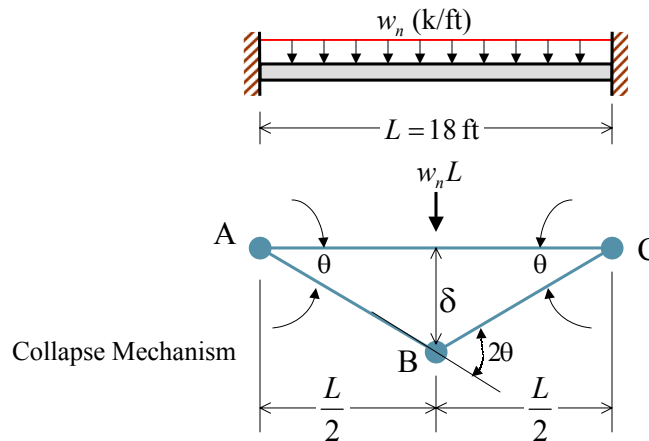
## Location of Plastic Hinge for Uniform Loadings

- Location of Plastic Hinge
  - For a uniformly loaded fixed-end beam shown in Fig. 1, the location of the plastic hinge along the length of the beam is at the midspan of the beam.
  - This was concluded due to the fact that beam is symmetrical in terms of both the uniform loading and the end supports.



## Location of Plastic Hinge for Uniform Loadings

Figure 1. Uniformly Loaded Fixed-end Beam



## Location of Plastic Hinge for Uniform Loadings

### ■ Location of Plastic Hinge (cont'd)

- For other beams with uniform loads, such as propped or continuous beams, the determination of the location of plastic hinge may be rather difficult.
- For this reason, a value, expressed as fraction of the length  $L$ , that determines the location of the plastic hinge is needed for the analysis of both the propped and continuous beams.



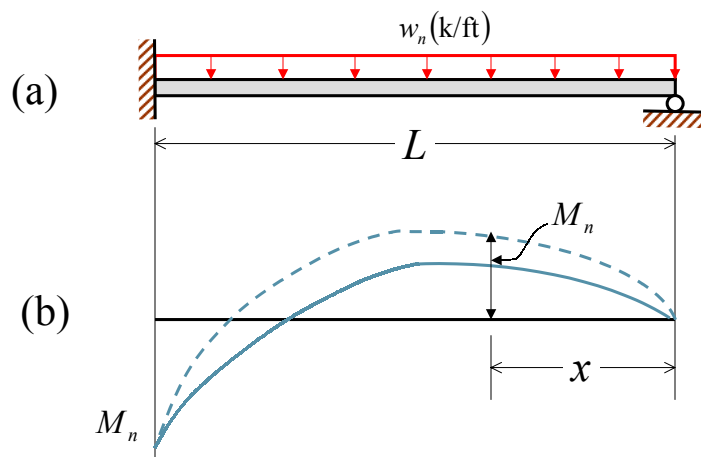
## Location of Plastic Hinge for Uniform Loadings

- Location of Plastic Hinge (cont'd)
  - Consider the propped beam of Fig. 2.
  - The elastic moment diagram for this beam is shown as the solid line in part (b) of the figure.
  - As the uniform load is increased in magnitude, a plastic hinge will first form at the fixed end.



## Location of Plastic Hinge for Uniform Loadings

Figure 2. Propped Beam





## Location of Plastic Hinge for Uniform Loadings

- Location of Plastic Hinge (cont'd)
  - At this time the beam will, in effect, be a “simple” beam with a plastic hinge on one end and a real hinge on the other.
  - Subsequent increases in the load will cause the moment to change as represented by the dashed line in part (b) of Fig. 2.



## Location of Plastic Hinge for Uniform Loadings

- Location of Plastic Hinge (cont'd)
  - The process will continue until the moment at some other point (a distance  $x$  from the right support in the figure) reaches  $M_n$  and create another plastic hinge.
  - The virtual-work expression for the collapse mechanism for the beam shown in Fig. 3 is written as follows:

$$[w_n L] \left[ \frac{1}{2} (\theta)(L-x) \right] = M_n \left( \theta + \theta + \frac{L-x}{x} \theta \right) \quad (1)$$

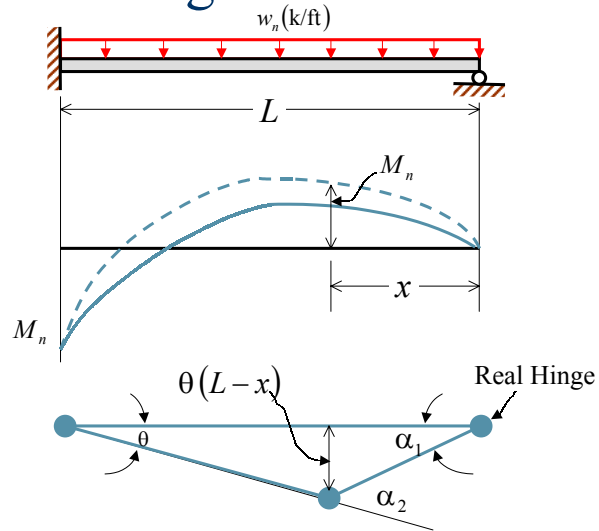


## Location of Plastic Hinge for Uniform Loadings

Figure 3.  
Collapse  
Mechanism

$$\alpha_1 = \frac{L-x}{x}\theta$$

$$\begin{aligned}\alpha_2 &= \theta + \alpha_1 \\ &= \theta + \frac{L-x}{x}\theta\end{aligned}$$



## Location of Plastic Hinge for Uniform Loadings

### ■ Location of Plastic Hinge (cont'd)

Or

$$M_n \left( \theta + \theta + \frac{L-x}{x}\theta \right) = [w_n L] \left[ \frac{1}{2}(\theta)(L-x) \right] \quad (2)$$

$$M_n = \frac{[w_n L] \left[ \frac{1}{2}(\theta)(L-x) \right]}{\left( \theta + \theta + \frac{L-x}{x}\theta \right)} \quad (3)$$

Solving for  $M_n$  by taking the derivative of  $M_n$  with respect to  $x$  and equate it to zero, that is

$$\frac{dM_n}{dx} = 0 \quad (4)$$



## Location of Plastic Hinge for Uniform Loadings

- Location of Plastic Hinge (cont'd)
  - Solving for  $x$ , it can be shown that Eq. 4 yields a value of  $x$  as given by

$$x = 0.414L \quad (5)$$

- This value is applicable to uniformly loaded end spans of both propped and continuous beams with simple supports.



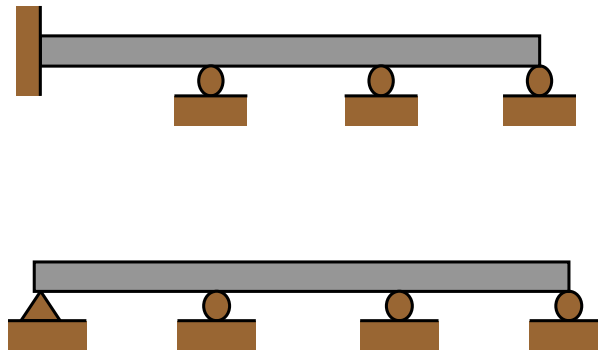
## Continuous Beams

- Continuous beams are very common in engineering structures (Fig. 4).
- They can be analyzed by both the elastic and the plastic theories.
- However, the plastic analysis can be more complicated.
- Plastic analysis can be applied to continuous beams as it is to one-span beams.



## Continuous Beams

Figure 4. Continuous Beams



## Continuous Beams

- The resulting values definitely give a more realistic picture of the limiting strength of a structure than can be obtained by elastic analysis.
- Continuous statically indeterminate beams can be handled by virtual-work method as they were for single-span statically indeterminate beams.



## Continuous Beams

### ■ Virtual-Work Method for Continuous Beams

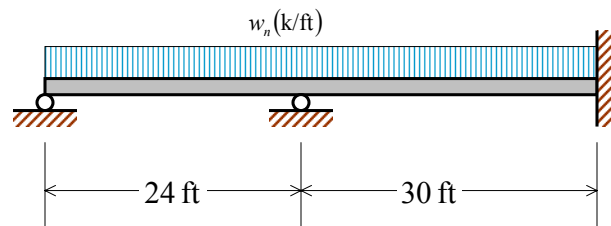
- For each span of the continuous beams, virtual-work expressions are written separately.
- From the resulting expressions, it is possible to determine the limiting or maximum loads that the beams can support.



## Continuous Beams

### ■ Example 1

A  $W18 \times 55$  ( $Z_x = 112 \text{ in}^3$ ) has been selected for the beam shown in the figure. Using 50 ksi steel and assuming full lateral support, determine the value of  $w_n$ .







## Continuous Beams

### ■ Example 1 (cont'd)

- The nominal (plastic) moment of the beam is calculated first:

$$M_n = F_y Z = \frac{50(112)}{12} = 466.7 \text{ ft} \cdot \text{k} \quad (6)$$

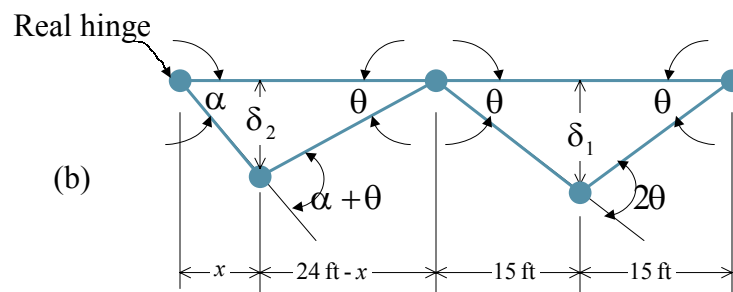
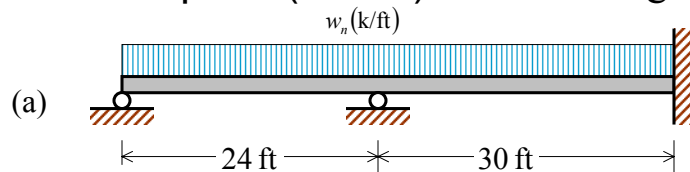
- The virtual-work expressions are written separately for each span of the continuous beams.
- The collapse mechanisms for the two spans are drawn as shown in Fig. 5b.



## Continuous Beams

### ■ Example 1 (cont'd)

Figure 5





## Continuous Beams

### ■ Example 1 (cont'd)

#### Calculation of rotation angles:

Using Eq. 4, the location of the plastic hinge for the left span is

$$x = 0.414L = 0.414(24) = 9.94 \text{ ft}$$

- From the triangle of the right span:

$$\tan\theta \approx \theta = \frac{\delta_1}{15} \Rightarrow \delta_1 = 15\theta$$

- From the triangle of the left span:

$$24 - x = 24 - 9.94 = 14.06 \text{ ft}$$



## Continuous Beams

### ■ Example 1 (cont'd)

- Also

$$\tan\theta \approx \theta = \frac{\delta_2}{14.06} \Rightarrow \delta_2 = 14.06\theta$$

- Therefore,

$$\tan\alpha \approx \alpha = \frac{\delta_2}{9.94} = \frac{14.06\theta}{9.94} = 1.414\theta$$

– Virtual-work applied to right span:

External work = Internal work

$$W_{\text{ext.}} = W_{\text{int.}}$$



## Continuous Beams

### ■ Example 1 (cont'd)

$$(w_n L_1) \times (\delta_1)_{\text{avg}} = M_n (\theta + \theta + 2\theta)$$

$$(w_n \times 30) \left( \frac{1}{2} \times 15\theta \right) = M_n (4\theta)$$

$$\therefore w_n = \frac{2(4M_n)}{30(15)} = 0.01778M_n = 0.01778(466.7) = 8.30 \frac{\text{k}}{\text{ft}}$$

Controls

8.30  $\frac{\text{k}}{\text{ft}}$ 

– Virtual-work applied to left span:

$$(w_n L_2) \times (\delta_2)_{\text{avg}} = M_n (\alpha + \theta + \theta)$$

$$(w_n \times 24) \left( \frac{1}{2} \times 14.06\theta \right) = M_n (1.414\theta + \theta + \theta)$$

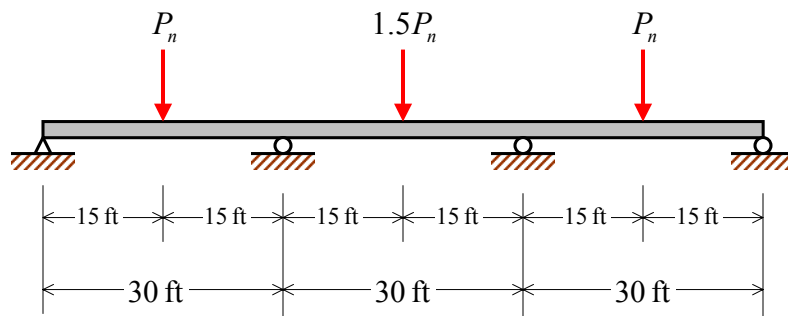
$$\therefore w_n = \frac{2(3.414M_n)}{24(14.06)} = 0.02023M_n = 0.02023(466.7) = 9.44 \frac{\text{k}}{\text{ft}}$$



## Continuous Beams

### ■ Example 2

Using a W21 × 44 ( $Z_x = 95.4 \text{ in}^3$ ) consisting of A992 steel, determine the value of  $P_n$  for the beam shown.



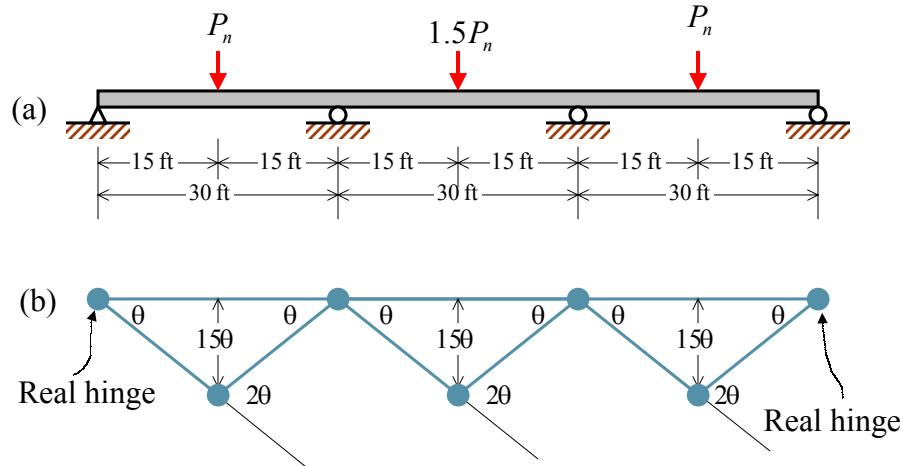


## Continuous Beams

### ■ Example 2 (cont'd)

Collapse Mechanisms:

Figure 6



## Continuous Beams

### ■ Example 2 (cont'd)

- The nominal (plastic) moment of the beam is calculated first:

$$M_n = F_y Z = \frac{50(95.4)}{12} = 397.5 \text{ ft} \cdot \text{k}$$

- The virtual-work expressions are written separately for each span of the continuous beams.
- The collapse mechanisms for the three spans are drawn as shown in Fig. 6b.



## Continuous Beams

### ■ Example 2 (cont'd)

For the first and third spans:

$$W_{\text{ext.}} = W_{\text{int.}}$$

$$P_n(150) = M_n(\theta + 2\theta)$$

$$P_n = \frac{M_n(3)}{15} = 0.2M_n = 0.2(397.5) = 79.5 \text{ kips}$$

For the center span:

$$W_{\text{ext.}} = W_{\text{int.}}$$

$$1.5P_n(150) = M_n(\theta + \theta + 2\theta)$$

$$P_n = \frac{M_n(4)}{1.5(15)} = 0.1778M_n = 0.1778(397.5) = 70.7 \text{ kips}$$

Controls



70.7 kips